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Highlights for “Comparing and combining physically-based and empirically-based approaches for estimating the hydrology of ungauged catchments” by Booker and Woods

- Methods for estimating various hydrological indices at ungauged sites were compared.
- Methods included a TopNet rainfall-runoff model and a Random Forest empirical model.
- TopNet estimates were improved through correction using Random Forest estimates.
- Random Forests provided the best estimates of all indices except mean flow.
- Mean flow was best estimated using an already published empirical method.
Title: Comparing and combining physically-based and empirically-based approaches for estimating the hydrology of ungauged catchments.

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Short title: hydrological estimates for ungauged catchments
Abstract

Predictions of hydrological regimes at ungauged sites are required for various purposes such as setting environmental flows, assessing availability of water resources or predicting the probability of floods or droughts. Four contrasting methods for estimating mean flow, proportion of flow in February, 7-day mean annual low flow, mean annual high flow, the all-time flow duration curve and the February flow duration curve at ungauged sites across New Zealand were compared. The four methods comprised: 1) an uncalibrated national-coverage physically-based rainfall-runoff model (TopNet); 2) data-driven empirical approaches informed by hydrological theory (Hydrology of Ungauged Catchments); 3) a purely empirically-based machine learning regression model (Random Forests); and 4) correction of the TopNet estimates using flow duration curves estimated using Random Forests. Model performance was assessed through comparison with observed data from 485 gauging stations located across New Zealand. Three model performance metrics were calculated: Nash-Sutcliffe Efficiency, a normalised error index statistic (the ratio of the root mean square error to the standard deviation of observed data) and the percentage bias. Results showed that considerable gains in TopNet model performance could be made when TopNet time-series were corrected using flow duration curves estimated from Random Forests. This improvement in TopNet performance occurred regardless of two different parameterisations of the TopNet model. The Random Forests method provided the best estimates of the flow duration curves and all hydrological indices except mean flow. Mean flow was best estimated using the already published Hydrology of Ungauged Catchments method.

Key words: hydrological indices; flow duration curves; ungauged sites; rainfall-runoff model; random forests.
River water provides a valuable resource for out-of-stream water use as well as for supporting in-stream environmental values. Alteration of natural river flow regimes is increasing globally as water is taken for human, agricultural and industrial use and power production, threatening both river biodiversity and security of human water use (Vörösmarty et al., 2010). Globally, this has led to a variety of legislative processes aimed at promoting prudent and rational use of natural water resources which seek to judge the trade-off between economic development and impact to the natural environment (e.g. EC, 2000; New Zealand Government, 2011). For example, default limits to water resource use for all rivers in New Zealand must comprise at least a minimum flow (the flow below which no water can be abstracted) and an allocation limit (a limit on the amount of abstraction taken from the resource) (New Zealand Government, 2011; Snelder et al., 2013).

Information summarising natural flow regimes is therefore required to assess both the in-stream environmental and out-of-stream economic effects of potential alterations to flow regimes. This information may take the form of various hydrological indices describing different aspects of the flow regime such as low flows, high flows or flow variability (Olden and Poff, 2003; Poff et al., 2010). Flow duration curves (FDCs) may also be utilised for various purposes including low flow analysis (Smakhtin, 2001), quantifying reliability of water supply (Snelder et al., 2011) and quantifying alterations to hydrological regimes (Vogel et al., 2007). This type of hydrological information is ideally derived from observed flow time-series at the site, or sites, of interest. However, flow time-series are only available at a small number of locations where flow gauges have been maintained and operated. Hydrological estimates are therefore often required at ungauged sites across a catchment or landscape (Sivapalan et al., 2003).
A variety of approaches can be used to provide estimates of hydrological indices at ungauged sites. In theory, these approaches range from purely physically-based to purely empirically-based. Physically-based approaches have also been referred to as deterministic (Chow et al., 1988), distributed (Beven and Binley, 1992), physics-based (Pechlivanidis et al., 2011), process-based or Newtonian (Yaeger et al., 2012). Empirically-based approaches have also been referred to as stochastic (Chow et al., 1988), metric (Pechlivanidis et al., 2011) data-based or Darwinian (Yaeger et al., 2012). Physically-based approaches are those that aim to estimate streamflow by utilising a conceptual understanding of the physics describing various parts of the hydrological cycle by approximating physical processes such as interception, evaporation, and storage (e.g. Beven and Kirkby, 1979; Clark et al., 2008). However, assumptions about physical processes are necessarily required to apply this understanding (Beven, 1997). For example, assumptions about continuity of volumes, discretisation of governing equations and some form of spatial averaging may be required for a physically-based approach to be spatially-distributed (Beven, 1989; Bloschl and Sivapalan, 1995; Singh and Frevert, 2006). Similarly, time dependence must be represented by updating state variables through a sequence of time steps (Singh, 1995). Physically-based approaches may also require spatially distributed input data such as information on soil characteristics such as water holding capacity, rainfall time-series or temperature time-series (e.g. Clark et al., 2008). This has led to much analysis and debate relating to data needs, parameter calibration and uncertainty in physically-based hydrological models (Beven, 1997; Beven, 1989; Singh and Woolhiser, 2002; Gupta et al., 2006).

Empirically-based approaches are those that seek to estimate hydrological indices by quantifying patterns between observed hydrological indices and catchment characteristics. These patterns can be quantified using a variety of techniques including linear regression (e.g. Engeland and Hisdal, 2009), or machine learning techniques (e.g. Booker and Snelder,
One advantage of empirically-based approaches is that their relative simplicity has allowed them to be transferred to ungauged catchments by way of regionalisation (e.g. Castellarin et al., 2004), generalisation or dissimilarity modelling (e.g. Booker and Snelder, 2012).

In practice, many physically-based models have empirical components and many empirical models incorporate some level of knowledge about physical processes. A balance between model complexity and data availability must be found for both physically-based (Fenicia et al., 2008) and empirically-based (Jakeman and Hornberger, 1993) approaches. All physically-based approaches require some parameterisation, and are known to perform best when calibrated against observed data (e.g. Clark et al., 2008; McMillan et al., 2013).

Similarly, the independent variables used in empirically-based approaches are often chosen after consideration of physical principles and the form of fitted empirical relationships can also be interrogated to ensure consistency with physical principles (e.g. Booker and Snelder, 2012). Hybrid metric-conceptual models are those that seek to combine the strengths of empirically-based and physically-based conceptual models (Pechlivanidis et al., 2011).

Despite the variety of approaches available for estimating hydrological conditions at ungauged sites, few studies have compared estimates calculated using contrasting approaches. The aim of this work was to compare a variety of available methods for estimating several hydrological indices and flow duration curves at ungauged catchments across New Zealand. These methods employed a range of approaches from a physically-based rainfall-runoff model to empirically-based regressions. The primary aim was to objectively judge which method was best able to estimate several hydrological indices across New Zealand given current climatic and landcover conditions. The secondary aim was to assess the advantages of combining two approaches by correcting physically-based estimated time-series using empirically-based estimated FDCs.
2. Data Description

a. Flow time-series

A flow time-series database was collated that comprised mean daily flows observed at 485 gauging stations with available records of 5 full years or longer. Available mean daily flow time-series from the National Institute of Water and Atmospheric Research’s (NIWA) national database were collated alongside data supplied by particular regional councils (Northland Regional Council, Auckland Council, Waikato Regional Council, Greater Wellington Regional Council, and Environment Canterbury). The time-series database contained only sites that were not affected by large engineering projects such as dams, diversions or substantial abstractions, according to information given by each data provider. See Snelder et al. (2005) and Booker (in press) for further details on gauging station selection. These gauging stations were located throughout New Zealand (Figure 1) and represented a wide range of hydrological conditions (Table 1). The observed time-series did not all cover the same time periods.

It is known that hydrological regimes may not be stationary (constant mean and constant variance through time; Hamilton, 1994) due to the presence of trends and temporal autocorrelations (Milly et al., 2008). This is because hydrological regimes may be influenced by a variety of factors including land cover change (e.g. Fahey & Jackson, 1997), inter-decadal climatic patterns (e.g. Kiem et al., 2003) and longer-term climate shifts (Parry et al., 2007). However, the purpose of this study was to compare the ability of various approaches to characterise differences in flow regimes between sites across New Zealand given current climatic and land cover conditions rather than to characterise differences through time. For empirically-based methods it was therefore assumed that differences in hydrological regimes between sites far exceeded any differences in hydrological regimes that may have occurred due to differences in observation periods (which were different for each observed time-series)
Despite some evidence for inter-decadal patterns in some, but not all, indices for particular regions of New Zealand but not others (e.g. McKerchar and Henderson, 2003; Booker, in press).

**b. Observed hydrological indices**

Several hydrological indices were calculated for each observed flow time-series (Table 2). These indices were chosen because they represent a range of hydrological conditions including floods and droughts, can be used to estimate water resource availability, and are used in environmental flow setting procedures. Mean flow, $Q_{\text{bar}}$, represents total potential water availability, is used for scaling of dimensionless metrics such as standardised flow duration curves (e.g. Booker and Snelder, 2012) and may be used when comparing sites for ecological studies (e.g. Leathwick et al., 2005). The proportion of flow in each month may be of interest when investigating seasonality of flow. The proportion of flow in February, $Q_{\text{Feb}}$, was chosen as an example because the mid-summer month of February represents a generally dry month in which both irrigation demand (the largest consumptive water use in New Zealand) and ecological stress are likely to be high. The 7-day mean annual low flow, $Q_{\text{MALF}}$, is often used as an indicator of low flow in ecological studies (e.g. Caruso, 2002; Suren and Jowett, 2006) and to represent one component of the flow regime in environmental flow assessments (e.g. Richter et al., 1997; Poff et al., 1997). Since limits to water resource use may be expressed as proportions of $Q_{\text{MALF}}$, this index is of particular interest in New Zealand (MFE, 2008). Mean annual flood, $Q_F$, may be used for flood risk assessment and flood design, but may also be used as a surrogate for physical disturbance (e.g. Poff and Ward, 1989; Poff, 1996) especially when compared to geomorphological characteristics such as sediment grain size and channel slope (Clausen and Plew, 2004). All four of these hydrological indices may also be used for data driven environmental classifications (e.g. Snelder and Booker, 2012). Many further hydrological indices could have
been compared, but it was desirable to provide an expedient analysis and there is known to be a high degree of covariance within sets of these indices (Clausen and Biggs, 1997; Olden and Poff, 2003).

In order to minimise the likelihood of low flow periods crossing years, each day in each observed time-series was assigned to a water year starting on the 1st of October. Water years with more than 30 days of missing data were excluded from the analysis. Calculations of \(Q_{\text{MALF}}\), and mean annual flood \(Q_F\) were based on water years. \(Q_{\text{MALF}}\) was calculated as being the mean of the 7-day running average annual low flow in each water year.

Many hydrological indices are scale-dependent; bigger catchments have larger values of \(Q_5\), \(Q_{\text{MALF}}\), \(Q_F\) and \(Q_{\text{bar}}\) than smaller catchments. The values for these indices were therefore standardised by dividing by catchment area. Further transformations were then applied in order to more closely approximate normal distributions (Table 2).

c. **Flow duration curves**

FDCs represent the relationship between magnitude and frequency of flow by defining the proportion of time for which any discharge is equalled or exceeded (Vogel and Fennessey, 1994; Vogel and Fennessey, 1995). Flow duration curves are a useful tool for quantifying flow regimes for both resource availability (Snelder et al., 2011) and for departure from a reference state (Vogel et al., 2007). For each flow time-series two observed FDCs were calculated from mean daily flows. FDCs were calculated from: a) mean daily flows in all months of the year; and b) mean daily flows in February. These two FDCs represent the probability distribution of flow over all-time and the probability distribution of flow for the month of February over all years. As above, February was chosen to represent a dry month in which both irrigation demand and ecological stress are likely to be high.

For calculation of each FDC, mean daily flows for each gauging station were sorted lowest to highest and then interpolated onto percentile values from 0 to 100 in intervals of 1
to determine the proportion of the time that each flow was not exceeded. Each FDC was therefore characterised using the same number of data points (101), providing for a balanced study design in further statistical analysis. All daily flows were divided by catchment area to allow modelling of differences in mean flow whilst standardising for differences in catchment size. This was in contrast to the method of Booker and Snelder (2012) which investigated only the shapes of FDCs after having standardised by $Q_{\text{bar}}$.

\textit{d. Catchment characteristics}

A GIS representation of the New Zealand river network comprising 550,000 segments, their unique upstream catchments and an associated database of catchment characteristics were used to provide information for each gauging station. The catchment characteristics include a range of categorical and continuous variables (Snelder and Biggs, 2002; Snelder et al., 2004; Leathwick et al., 2011). The GIS river network and associated databases have previously been used to define a hierarchical classification of New Zealand’s rivers called the River Environment Classification (REC; Snelder and Biggs 2002). These databases provide inventories for river resource analysis and management purposes (Snelder and Hughey, 2005; Leathwick et al., 2011; Clapcott et al., 2010; Clapcott et al., 2011). They have also been used to create nationwide models for estimating flow statistics such as flood flows (Pearson and McKerchar, 1989), low flows (Pearson, 1995), mean flow (Woods et al., 2006) and shapes of FDCs (Booker and Snelder, 2012) at ungauged sites using relationships between these hydrological metrics and catchment characteristics. Snelder et al. (2005) showed that grouping river segments by nested categorical subdivisions of climate and topography, known as the Source-of-Flow grouping factor (Table 3), provided an a priori hydrological regionalisation.
3. **Estimation methods**

For this study four methods for calculating hydrological indices and FDCs at ungauged locations were compared (Figure 2). Method 1 used a physically-based approach. Method 2 used a data-driven empirical approach that was informed by hydrological theory to estimate each hydrological index separately. Method 2 can be classified as being a hybrid metric-conceptual approach under the classification proposed by (Pechlivanidis et al., 2011). Method 2 was named after a sequence of projects collectively known as the Hydrology of Ungauged Catchments (HUC) projects. Method 3 used an empirically-based regression approach. Method 4 combined a physically-based and empirically-based approach. All methods were able to produce estimates for all reaches that comprise the NZ river network and were therefore applicable to ungauged sites across New Zealand.

a. **Method 1 TopNet**

TopNet is a spatially distributed time-stepping hydrological model which combines TOPMODEL concepts of sub-surface storage controlling the dynamics of the saturated contributing area and baseflow recession (Beven and Kirkby, 1979; Beven et al., 1995) with submodels for snow and plant canopies, and a kinematic wave channel routing algorithm (Goring, 1994). See McMillan et al. (2013) for further detailed description and Clark et al. (2008) for complete model equations.

TopNet has two fundamental components: (i) simulating the water balance over sub-catchments throughout a river basin, and (ii) routing streamflow from each sub-catchment to the basin outlet. The water balance model includes simulating the storages and fluxes of water in the canopy, snowpack, unsaturated and saturated soil zone. TopNet also accounts for time delay due to flow routing within each sub-basin. Runoff from each sub-basin flows into a digital stream network and is routed through the river network. For this application TopNet models used daily precipitation and temperature data from the New Zealand Virtual Climate
Station Network (Tait, 2008, Tait et al., 2006), which was then disaggregated to hourly resolution using stochastic disaggregation for precipitation (Rupp et al., 2009). Additional model boundary conditions were estimated directly from GIS data on topography, soil and vegetation (Clark et al., 2008; McMillan et al., 2013).

For catchment specific applications TopNet parameters can be calibrated to optimise model performance (e.g. Bandaragoda et al., 2004; McMillan et al. 2013). However, in this case uncalibrated national TopNet models of New Zealand (Henderson et al., 2011) were run using an hourly timestep over the period 1973-2010. Two different versions of TopNet were available. National TopNet Version 0 was discretised using Strahler-1 sub-catchments from the REC. The typical catchment area of a Strahler-1 catchment is 0.7 km². This version had a spatially uniform value for the parameter, $f$, which represents the decline in saturated hydraulic conductivity of the soil with depth (Clark et al., 2008). This parameter effectively controls responsiveness of river flow to rainfall. National TopNet Version 1 was discretised using Strahler-3 sub-catchments from the REC. This version had a spatially distributed set of values for $f$. The $f$ parameter took different values according to the hydrological regionalisation described by Toebes and Palmer (1969), ranging from values more than 8 $m^{-1}$ for steep catchments in the Southern Alps to less than 1 $m^{-1}$ in flat catchments on the volcanic plateau in the central North Island (see Figure 1 for place names). Where flow time-series were required for Strahler-1 and Strahler-2 catchments flow data were downscaled by multiplying flows from the nearest available Strahler-3 node in the REC network by the ratio of the catchment area of the required location with that of the substitute location. For both Version 0 and Version 1 hourly data for the river reach in which each gauging station was located were averaged over each calendar day to obtained mean daily flow time-series. Hydrological indices were then calculated using the same algorithms as were applied to the observed flow time-series.
Ideally both observed and estimated time-series would be available for a very long period (e.g. 100 years). However, the available observed flow time-series did not all cover the same period, and TopNet data were available for a uniform time period (1973-2010). This provided the opportunity to test the sensitivity of correspondence between observed and estimated hydrological indices to synchronisation of the observed and TopNet estimated time-series. Observed and TopNet Version 1 estimated indices were compared using two different procedures. For the first procedure, indices calculated from all available observed flows (5 years or more) were compared with those calculated from all available TopNet Version 1 estimated flows (1973-2010). Essentially this procedure assumed that, when averaged over time, both the observed and TopNet estimated time-series represented the long term hydrological conditions (i.e. that both observed and TopNet estimated time-series were stationary and that records were sufficiently long to characterise long term conditions). For the second procedure only the time period for which both observed flows and TopNet estimated flows were available was identified for each gauging station. Observed indices for this period were then compared with TopNet Version 1 estimated indices for the same period at each gauged location. Better fit between synchronised observed and estimated values (the second procedure) in comparison to non-synchronised (the first procedure) would indicate non-stationarities in the observed hydrological regimes that were detectable in the TopNet time-series. Some observed time-series fell completely outside of the TopNet time-series. This reduced the number of time-series available for the second procedure compared to the first.

**b. Method 2 HUC**

The approach used to estimate $Q_{\text{bar}}$ for Method 2 (HUC) is described in Woods et al. (2006). Woods et al. (2006) evaluated four simple models of mean annual runoff throughout New Zealand, predominantly based on precipitation information and estimated
evapotranspiration. Model results were compared to observed data and synthesised estimates of catchment runoff. The preferred model of Woods et al. (2006) subtracts an estimate of annual actual evapotranspiration from a precipitation surface. Annual actual evapotranspiration is estimated according to the ratios of potential evapotranspiration with annual precipitation, and a single water balance parameter which is estimated by independent calibration. This method applies a regional bias correction to the results of a previously uncorrected model.

The approach used to estimate $Q_{\text{Feb}}$ for Method 2 was to employ a regionalisation of $Q_{\text{Feb}}$ based on Source-of-Flow groupings in the REC and New Zealand island (i.e. North Island or South Island, Figure 1), where Source-of-Flow is a combination of the climate and topography classes of a catchment (Table 3). For each region $Q_{\text{Feb}}$ was the mean of the $Q_{\text{Feb}}$ for all observed flow records that belong to that class in that island. For cases where no measured flow was available, expert judgement was applied to make use of data from other classes.

The approach used to estimate $Q_{\text{MALF}}$ for Method 2 is described in Henderson et al. (2004). Figure 3 shows a schematic description of the model and its parameters. These fall into three categories: a) climate parameters ($T$ the average length of a dry season, $N$ the number of rain events in that season, $P$ the amount of rain in the dry season); b) flow parameters ($Q_{\text{mean}}$ the mean flow, $Q_0$ the average flow at the start of the dry season, $\alpha$ the fraction of that rain that affects the streamflow); and c) catchment parameters that describe the way in which water is released from catchments during the dry season ($b$ and $T^*$).

Estimates of all these input parameters have previously been developed for all of New Zealand (Henderson et al, 2004). The parameter $Q_0$ corresponds to the average flow at the start of the dry season. The predictions are most sensitive to the value of the $b$ parameter, which describes the type of river flow recession. For example, catchments in dry catchments...
typically have $b$ values near 1, hill country catchments typically have $b$ values near 2, and catchments with volcanic geology typically have $b$ values of 3 or larger.

The approach used to estimate $Q_F$ for Method 2 is described in Pearson and McKerchar (1989) and McKerchar and Pearson (1989). Essentially, these estimates are gained from interpolation onto ungauged sites from a contour map of $Q_F$ which was itself derived from a spatial interpolation of observed data. Since this approach used instantaneous flow data to calculate $Q_F$, rather than mean daily values, it was anticipated that the approach would overestimate $Q_F$ in comparison to observed values derived from mean daily values. However, the estimates were still included in the analysis.

The approach used to estimate FDCs for Method 2 was to assume a log-normal probability distribution as a model of the flow duration curves. This is a log transformation of

$$g(x, \theta) = \frac{1}{\sqrt{2\pi}\sigma} \exp\left[-\frac{1}{2}\left(x - \theta_1\right)^2\sigma^2\right],$$

Equation 1

which has two parameters, $\theta_1$ and $\theta_2$. It was further assumed that $\theta_1$ could be estimated as the mean flow ($Q_{\text{bar}}$ from Method 2) and that $\theta_2$ would be estimated as a linear function of the $b$ parameter, which was also used to calculate $Q_{\text{MALF}}$ for Method 2. The approach used to estimate $FDC_{\text{Feb}}$ was to scale the estimated FDC for Method 2 by the estimated $Q_{\text{Feb}}$ for Method 2.

c. Method 3 Random Forests

A regression technique called Random Forests was used to apply a regression of each observed hydrological index (Table 2) and each of the three parameters describing a GEV distribution of the all-time FDC and the FDC for February as a function of available catchment characteristics (Table 4). This method uses machine-learning by combining many regression trees into an ensemble to produce more accurate regressions by drawing several bootstrap samples from the original training data and fitting a tree to each sample (Breiman, 2001; Cutler et al., 2007). Random forest models fitted using catchment characteristics have
previously been shown to be able to explain variation in hydrological patterns such as parameters describing FDCs (Booker and Snelder, 2012), the frequency of events that exceed three time the median flow (Booker, in press) and various other hydrological indices (Snelder and Booker, 2012). Each random forest was developed by growing 500 trees. As the number of trees (k) increases the generalisation error always converges and it was assumed that use of 500 trees was sufficiently high to ensure convergence.

The predictions from random forest models were tested using a leave-one-out cross validation procedure referred to here as jack-knifing (Efron, 1982; Booker and Snelder, 2012). This cross-validation procedure was applied by leaving out all data associated with each of the 485 sites and then estimating each hydrological index for the left-out site from all remaining sites. The results from this procedure produced estimates as if each site were ungauged (Ganora et al., 2009). Comparison between observed and jack-knifed values allowed an assessment of both the robustness and reliability for estimation at ungauged sites (Castellarin et al., 2004).

For each time-series, the parameters describing a GEV distribution,

\[ G(x, \theta) = \exp \left[ -\left( 1 - \left( \frac{x - \theta_2}{\theta_1} \right) / \theta_0 \right)^\theta \right], \tag{Equation 2} \]

were fitted to all observed mean daily flows and all observed mean daily flows in February. In both cases observed mean daily flows were divided by catchment area for each gauging station prior to fitting the GEV parameters. The GEV distribution is described by three parameters and has shown to represent the range of shapes of standardised FDCs found across New Zealand. See Booker and Snelder (2012) for further discussion of estimating standardised FDCs at ungauged sites across New Zealand using various statistical techniques to generalise parameters describing various probability distributions.
Method 4 TopNet Corrected

FDCs calculated using the jack-knifed Random Forests method represent a unique FDC at any location in the New Zealand river network as if each location were ungauged. This provided the opportunity to correct for bias in the TopNet estimated FDCs using the Random Forests estimated FDC at each site as if it were an observed FDC. Therefore the jack-knifed Random Forests FDCs were used to calculate a correction factor for each percentile, \( i \), of the TopNet FDC for each site, \( j \).

\[
\text{TopNet Corrected}_{ij} = \text{TopNet}_{ij} \times \frac{\text{Random Forest}_{ij}}{\text{TopNet}_{ij}} \quad \text{Equation 3}
\]

Since the exceedance percentile of each datum in each TopNet time-series was known, these corrections could also be applied to each TopNet time-series. This allowed re-calculation of each hydrological index from each corrected time-series. This procedure was repeated separately for TopNet Version 0 FDCs and TopNet Version 1 FDCs.

4. Observed versus predicted values

Scatterplots of observed versus predicted values after having standardised and transformed each index (Table 2) were plotted for each index for each method. These scatterplots were overlaid with a linear regression with observed values on the y-axis as recommended by Piñeiro et al. (2008). Following the suggestion of Moriasi et al. (2007), three model performance metrics were calculated for each set of observed versus predicted values: Nash-Sutcliffe efficiency (NSE); percent bias (pbias); and ratio of the root mean square error to the standard deviation of observed data (RSR). NSE is a dimensionless metric that determines the relative magnitude of the residual variance (“noise”) compared to the observed data variance (“information”) (Nash and Sutcliffe, 1970). NSE values of 1 indicate a perfect match between estimates and observations, whereas values of 0 indicate performance equal to estimating the mean observed value across all observations. pbias measures the average tendency of the simulated data to be larger or smaller than their
observed counterparts (Gupta et al., 1999). Negative pbias values represent overestimation and positive values indicate underestimation. RSR standardises RMSE using the observations standard deviation, and it combines both an error index and the additional information recommended by Legates and McCabe (1999). Lower RSR values indicate better model performance, with 0 indicating perfect correspondence between estimates and observations. See Moriasi et al. (2007) and references therein for full details of these performance evaluation metrics. The same metrics were applied to 101 points representing log specific (flow per unit catchment area) FDCs for each site for each method for the February and all-time FDCs separately.

5. Results

a. Hydrological indices

Synchronisation of TopNet Version 1 with the observed time-series made little impact on the performance metrics (NSE, RSR and pbias) when compared to using the full TopNet time-series (Table 5). This was especially the case for $Q_{\text{bar}}$, $Q_{\text{MALF}}$ and $Q_F$. For $Q_{\text{bar}}$, synchronisation marginally reduced an overestimation bias, but also resulted in a small reduction in performance in terms of NSE and RSR (reduced NSE, increased RSR). For $Q_{\text{MALF}}$, synchronisation resulted in increased overprediction bias, but marginally improved performance in terms of NSE and RSR. The process of synchronisation did alter performance for $Q_F$ as synchronisation improved performance in terms of NES and RSR, but substituted an overprediction bias with an underprediction bias of the same magnitude. These results indicate that it was not the case that there were non-stationarities in observed hydrological regimes that were generally detectable in the TopNet time-series for $Q_{\text{bar}}$, $Q_{\text{MALF}}$ or $Q_F$. This may not have been the case for $Q_{\text{Feb}}$. This is an understandable result as $Q_{\text{bar}}$, $Q_{\text{MALF}}$ and $Q_F$ will be less sensitive to inter-annual variability than $Q_{\text{Feb}}$. This is because $Q_{\text{bar}}$ is an average calculated over all the record, and both $Q_{\text{MALF}}$ and $Q_F$ are both averages of indices calculated
for each year of record, whereas $Q_{Feb}$ is calculated over a smaller time-window in each year of record. Overall there was more difference in performance between TopNet Version 0 and TopNet Version 1 than there were differences between synchronisation and non-synchronisation of TopNet Version 1. This indicates that TopNet results are more sensitive to changes to the TopNet $f$ parameter than to either the assumption that the 1973-2010 time-series represent the long-term flow regime, or any non-stationarities combined with relatively short records in the observed time-series.

When compared to TopNet Version 0, TopNet Version 1 reduced an overestimation of $Q_{bar}$, but reduced performance in terms of NSE and RSR. For $Q_{Feb}$, TopNet Version 1 marginally improved NSE, reduced an overestimation p bias, but increased RSR. For $Q_{MALF}$, TopNet Version 1 dramatically improved NSE, improved RSR and replaced a large overestimation with an underestimation of lesser magnitude. For $Q_{F}$, TopNet Version 1 reduced performance of all metrics when compared to TopNet Version 0. This indicates that high flows were not better predicted following the regionalisation of the TopNet $f$ parameter. However, over all four indices there were greater differences between methods (TopNet, HUC and Random Forests) than there was between the two TopNet versions (Table 5, Figure 4).

The TopNet time-series was corrected using the jack-knifed Random Forests FDC estimates and then used to estimate the hydrological indices. For all indices and both TopNet versions, corrected estimates improved performance in terms of NSE and RSR when compared to the uncorrected TopNet estimates. Corrected estimates produced less bias as indicated by smaller magnitude p bias when compared to uncorrected estimates from both TopNet versions for all indices except $Q_{Feb}$ for Version 1 and $Q_{F}$ for version 0. Correction of TopNet Version 1 caused an increase in overprediction of $Q_{Feb}$. Correction of TopNet
Version 0 caused an overprediction to change to an underprediction of greater magnitude. Overall, correction greatly reduced differences in performance between the two TopNet versions (Table 5, Figure 4).

For $Q_{\text{bar}}$ and $Q_{\text{Feb}}$ there was more difference between TopNet Version 0 and TopNet Version 1 than there was between TopNet Version 1 and TopNet 1 Corrected. After correction, the performance of $Q_{\text{bar}}$ estimated from both TopNet versions matched the performance of those estimated using Random Forests. This was because the correction procedure forced the TopNet corrected estimated FDCs to match jack-knifed Random Forests estimated FDCs and therefore TopNet corrected $Q_{\text{bar}}$ matched jack-knifed Random Forests estimated $Q_{\text{bar}}$.

NSE was positive (negative values indicate that the mean observed value is a better predictor than the simulated value) for all indices for all methods except $Q_F$ for Method 2 HUC (Table 5). This indicates that, except for $Q_F$ from the HUC method, all methods provided some degree of useful information about patterns in the estimated values. In this comparison HUC estimates of instantaneous $Q_F$ were compared with observed $Q_F$ calculated from mean daily flow data. Poor performance and, in particular, overestimation of $Q_F$ for Method 2 HUC was therefore not surprising. In fact, McKerchar and Pearson (1989) previously showed that the method was able to explain a substantial fraction of the observed variation in $Q_F$ when compared to observed values calculated from instantaneous flow data.

For $Q_{\text{bar}}$ the HUC method performed best in terms of both NSE and RSR. This is the method already recommended by Woods, et al. (2006). For $Q_{\text{MALF}}$, $Q_F$ and $Q_{\text{Feb}}$ the Random Forests method performed best in terms of both NSE and RSR. The Random Forests method also gave the lowest magnitude pbias for $Q_F$ and $Q_{\text{Feb}}$ but not for $Q_{\text{MALF}}$ (Table 5). These findings correspond well with visual inspection of observed against predicted values, which
indicated that the Random Forests method reduced scatter and produced unbiased estimates for all four indices but was out-performed by Method 2 HUC for $Q_{bav}$ (Figure 4).

b. Flow duration curves

More sites had better performance as indicated by higher NSE values, lower RSR values and lower magnitude pbias for all-time FDCs compared to February FDCs regardless of estimation method (Figure 5). This indicates greater uncertainties associated with estimation of February FDCs compared to all-time FDCs. More sites had better performance in terms of NSE, RSR and pbias for TopNet Version 1 in comparison to TopNet Version 0 for the all-time FDC and the February FDC in particular. Negative pbias values for many TopNet Version 0 estimated February FDCs indicated consistent underestimation. This consistent underestimation was not present for TopNet Version 1, which showed an equal likelihood for either underestimation or overestimation of the February FDC. This indicated that regionalisation of the TopNet $f$ parameter improved flow estimation, particularly in February.

Both the HUC and the Random Forests methods performed better than either of the uncorrected TopNet methods for both the all-time and February FDCs. Both all-time and February FDCs had more sites with higher NSE, lower RSR and lower magnitude pbias when estimated using the Random Forests method compared to the other methods. Since the TopNet 1 Corrected estimated all-time FDC was corrected using the jack-knifed Random Forests estimated FDC, performance of the TopNet 1 Corrected estimated all-time FDC was the same as the jack-knifed Random Forests estimated FDC.

c. National estimates for New Zealand

All methods were able to provide predictions for ungauged sites across New Zealand which reproduced the major regional variations in observed $Q_{MALF}$ (Figure 6). These geographical patterns included a strong east-west gradient in the South Island as well as the
influence of the Southern Alps (see Figure 1 for place names). As they cross the eastern plains of the South Island, large mountain-fed rivers with markedly higher $Q_{MALF}$ stand out against a background of comparatively lower-yielding lowland streams. To the northeast of the central North Island, the rivers draining a volcanic plateau have relatively high $Q_{MALF}$, with large storage capacity in the thick pumice and ash layers sustaining low flows (Mosley and Pearson, 1997). Both Random Forests (Figure 6c) and TopNet (Figure 6d) predicted lower values of $Q_{MALF}$ than HUC (Figure 6b) for the south west coast of the South Island, but predicted slightly higher $Q_{MALF}$ for most other locations in comparison with HUC. It should be noted that none of the methods were designed to take account of large engineering schemes such as those currently in place on several of New Zealand’s large rivers (e.g. the Waikato, Rangitata, Waitaki, Clutha and Waiau rivers).

6. Discussion

A limited set of hydrological indices along with both the all-time and February FDCs were investigated (Table 2). This set of hydrological indices included those representing both high and low flow extremes as well as an aspect of seasonality. These indices are commonly used for water resource planning in New Zealand, however not all aspects of the flow regime, such as the frequency of mid-range flows, were represented. This aspect of the flow regime could have been included by calculating various additional indices such as the number of events exceeding three times the long-term median flow (FRE3; Biggs 2000), but no HUC method was available for estimating this index. National estimates of FRE3 using random forests, including comparison with observed values, were calculated and compared with observations by Booker (in press).

For the Random Forests method FDCs were described using the three parameter GEV distribution. Other distributions could have been used including log Pearson Type III (LP3; Ganora et al., 2009) or a mixed gamma distribution (Cheng et al., 2012). Booker and Snelder
(2012) showed that, although the LP3 distribution may provide better fits to observed FDCs when standardised by mean flow, uncertainties in generalising the LP3 parameters from catchment characteristics meant that a method using the GEV distribution to parameterise the shape of the FDC gave better performance for prediction at ungauged locations.

The same set of independent variables was used to model all four hydrological indices. Procedures designed to optimise the set of independent variables such as the Model Improvement Ratio (Murphy et al. 2010) were not employed to optimise the predictor data set. This approach may not have provided optimal Random Forest models in all cases as one would expect different sets and different numbers of independent variables to best predict each dependent variable. For example, summer temperature might be expected to be related to low flows, but not flood flows. Despite this the Random Forests method still outperformed the other methods even when a leave-one-out cross validation procedure was applied to allow for independent assessment of estimation performance against observed data.

Although many performance metrics are available to assess model performance, NSE, RSR and pbias were used as recommended by Moriasi et al., (2007). Although these three metrics are designed to quantify different aspects of model performance, they often gave consistent information regarding model performance.

The aim of this work was to assess the ability of various methods to estimate hydrological conditions for ungauged catchments in the absence of major hydrological alterations such as that caused by abstraction, storage or diversion. The ability to estimate the effects of either climate change or land cover change were not assessed. It may be necessary to assess the potential effects of climate change (Zemansky et al., 2012; Earman and Dettinger 2011), land use change (Scanlon et al., 2007) or their combined effects (Brekke et al., 2004) on flow regimes to develop rational management strategies. Both TopNet and the Random Forests models described above have inputs that could be changed to assess the
impacts of climate change. However, the validity of this approach was not tested here. It should be noted that there are several issues relating to model structure and parameterisation that would need to be resolved when using physically-based models to predict the hydrological impacts of environmental change (Wagener, 2007). Similarly, when using flexible empirically-based models such as Random Forests to predict outside of the fitted model domain it is important to understand how the algorithms perform when projected into the new environmental conditions (Elith and Graham, 2009).

These results indicate that Random Forests outperformed both TopNet versions for all four hydrological indices as well as for FDCs. This finding corresponds well with the findings of others. For example, Parkin et al. (1996) found that streamflow predictions from an a priori parameterised physically-based model contained considerable uncertainty. It should be noted that, although TopNet Version 1 arguably represents the best currently available physically-based approach for application to ungauged sites across New Zealand, this method was uncalibrated. It is known that calibration of TopNet parameters can significantly improve estimation performance by optimising model performance against observed flows (e.g. Bandaragoda et al., 2004; McMillan et al., 2013). Calibration procedures are only possible for catchment specific applications with available flow data. It is possible to transfer calibrated parameter sets to ungauged sites (e.g. Yu and Yang, 2000) given a suitable regionalisation procedure (McDonnell and Woods, 2004; Li et al., 2010; Olden et al., 2012; Coopersmith et al., 2012). Although calibration procedures have been applied to TopNet for several catchments (Bandaragoda et al., 2004; Clark et al., 2008; McMillan et al. 2013), a procedure to regionalise the calibrated parameter values is not currently available. Such procedures can be hampered by issues such as equifinality within the calibration parameter sets (Beven 2006; Bárdossy, 2007).
The Random Forests method can be used to estimate a unique FDC at any location in the New Zealand river network. These estimated FDCs could be used to provide a more reliable regionalisation than would be the case using data from observed locations alone because they represent variability across all of New Zealand rather than a sample of observed FDCs (Snelder and Booker, 2012). Furthermore, the Random Forests estimated FDC’s at ungauged locations could provide the opportunity to calibrate TopNet parameters against an estimated FDC for ungauged locations in the New Zealand river network. This would require a method that allowed calibration against an observed (or estimated) FDC (e.g. Yu and Yang 2000; Yadav et al., 2007; Westerberg et al., 2011). Such a method may be developed as part of future work. However, considerable improvements in performance were gained when both TopNet versions were corrected using the jack-knifed estimated FDCs from Random Forests. This indicates that TopNet performance can be increased considerably without automated parameter set calibration procedures (Yu and Yang, 2000) or increased understanding of hydrological processes controlling variability of FDCs across catchments (Yaeger et al., 2012). Furthermore, the correction procedure reduced differences in performance between TopNet Version 0 and TopNet Version 1.

The TopNet correction procedure tested here represents one relatively crude method of combining a process-based approach with a data-based approach. The procedure provides estimates calculated using a data-based approach to correct for bias within FDCs calculated using a process-based approach. This contrasts with alternative approaches which have augmented stochastic approaches with more process-based approaches by incorporating different components of catchment dynamic responses into stochastic models (e.g. Botter et al., 2007a, 2007b, 2009; Muneepeerakul et al., 2010; Cheng et al., 2012) or by applying a water balance modelling framework to divide the FDC into three parts (Yokoo and Sivapalan, 2011).
The TopNet correction procedure provided results that matched the performance of Random Forests for $Q_{bar}$ and the all-time FDC, but not for $Q_{Feb}$, $Q_{MALF}$ or $Q_F$. It should be noted that this procedure allowed improved estimation of the entire time-series of flows using both TopNet versions. This method has a major advantage over the Random Forest method because any required hydrological indices can be calculated from the estimated time-series. In contrast, the Random Forests method requires fitting of new models to any newly calculated indices prior to estimation of these new indices at ungauged sites.

7. Conclusion

Results showed the Random Forests method provided the best estimates of both FDCs and all four hydrological indices except mean flow. Mean flow was best estimated using the already published HUC method (Woods et al., 2006). Results also showed that considerable gains in estimation performance can be made by correcting estimates calculated using physically-based models with estimated values calculated using empirically-based models.

8. Acknowledgements

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Tables

Table 1. Codes, descriptions and numbers of sites used in the analysis. See Snelder and Biggs (2002) and Snelder and Hughey (2005) for full descriptions of codes.

Table 2. Hydrological Indices derived from observed mean daily flows.

Table 3. Summary of the defining characteristics, categories and category membership criteria that combine to define Source-of-Flow groupings within the REC.

Table 4. Codes and descriptions of independent variables used to fit regression models. See Leathwick et al., (2011) for full descriptions.

Table 5. Various metrics quantifying correspondence between observed and predicted values for four hydrological indices (Table 2) using various estimation methods.
Figures

Figure 1. Map showing the locations of the gauging stations used in this study.

Figure 2. Schematic showing different methods used to estimate hydrological indices and flow duration curves (FDCs).

Figure 3. Hydrology of ungauged catchments (HUC) low flow model and parameters.

Figure 4. Observed against calculated values for each index for each method (n = 485). Grey dashed line is linear regression. Black line is 1:1 such that x-limits are equal to y-limits for all plots. Qbar is mean flow. QFeb is proportion of flow in February. QMALF is 7-day mean annual low flow. QF is mean annual flood.

Figure 5. Box and whisker plots of Nash-Sutcliffe efficiency, RSR (ratio of the root mean square error to the standard deviation of observed data) and pbias (average tendency of the calculated data to be larger or smaller than their observed counterparts) at each site for all-time and February flow duration curves for each method (n = 101 points at each of 485 sites).

Figure 6. All observations and for each method predictions of 7-day mean annual low flow (MALF) for all rivers of Strahler order greater than three. TopNet results are for uncorrected TopNet Version 1.
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<table>
<thead>
<tr>
<th>Code</th>
<th>Description</th>
<th>Number of sites, total</th>
</tr>
</thead>
<tbody>
<tr>
<td>Island</td>
<td></td>
<td></td>
</tr>
<tr>
<td>N</td>
<td>North Island</td>
<td>289</td>
</tr>
<tr>
<td>S</td>
<td>South Island</td>
<td>196</td>
</tr>
<tr>
<td>Climate</td>
<td></td>
<td></td>
</tr>
<tr>
<td>WD</td>
<td>Warm-dry</td>
<td>18</td>
</tr>
<tr>
<td>WW</td>
<td>Warm-wet</td>
<td>152</td>
</tr>
<tr>
<td>WX</td>
<td>Warm-extremely wet</td>
<td>4</td>
</tr>
<tr>
<td>CD</td>
<td>Cool-dry</td>
<td>75</td>
</tr>
<tr>
<td>CW</td>
<td>Cool-wet</td>
<td>154</td>
</tr>
<tr>
<td>CX</td>
<td>Cool-extremely wet</td>
<td>82</td>
</tr>
<tr>
<td>Topographic source of flow</td>
<td></td>
<td></td>
</tr>
<tr>
<td>GM</td>
<td>Glacial mountain</td>
<td>10</td>
</tr>
<tr>
<td>H</td>
<td>Hill</td>
<td>167</td>
</tr>
<tr>
<td>L</td>
<td>Low elevation</td>
<td>241</td>
</tr>
<tr>
<td>Lk</td>
<td>Lake</td>
<td>19</td>
</tr>
<tr>
<td>M</td>
<td>Mountain</td>
<td>48</td>
</tr>
<tr>
<td>Land cover</td>
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<td></td>
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<td>B</td>
<td>Bare</td>
<td>16</td>
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<td>Exotic-Forest</td>
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<td>IF</td>
<td>Indigenous-Forest</td>
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<td>Pastoral</td>
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<td>S</td>
<td>Scrub</td>
<td>17</td>
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<td>T</td>
<td>Tussock</td>
<td>63</td>
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<tr>
<td>U</td>
<td>Urban</td>
<td>15</td>
</tr>
</tbody>
</table>
Table 2. Hydrological Indices derived from observed mean daily flows.

<table>
<thead>
<tr>
<th>Index</th>
<th>Description</th>
<th>Calculation</th>
<th>Standardisation</th>
<th>Transformation</th>
</tr>
</thead>
<tbody>
<tr>
<td>$Q_{\text{bar}}$</td>
<td>Mean flow over all time</td>
<td>Mean of all daily flows</td>
<td>Divide by catchment area to get specific mean flow ($\text{m}^3 \text{s}^{-1} \text{km}^{-2}$)</td>
<td>Log base 10</td>
</tr>
<tr>
<td>$Q_{\text{Feb}}$</td>
<td>Proportion of flow in February</td>
<td>Mean of all daily flows for each calendar month after having divided by the overall mean flow</td>
<td>Divide by mean flow over entire record to get proportion of flow in February (unitless)</td>
<td>None</td>
</tr>
<tr>
<td>$Q_{\text{MALF}}$</td>
<td>Mean of minimum 7-day flow in each year</td>
<td>Mean of minimum flow for each water year after having applied a running 7-day mean to the daily flows</td>
<td>Divide by catchment area to get specific $Q_{\text{MALF}}$ ($\text{m}^3 \text{s}^{-1} \text{km}^{-2}$)</td>
<td>Square root</td>
</tr>
<tr>
<td>$Q_{\text{F}}$</td>
<td>Mean of maximum flow in each year</td>
<td>Mean of maximum flow for each water year</td>
<td>Divide by catchment area to get specific $Q_{\text{F}}$ ($\text{m}^3 \text{s}^{-1} \text{km}^{-2}$)</td>
<td>Log base 10</td>
</tr>
<tr>
<td>FDC</td>
<td>Probability distribution of daily flow</td>
<td>Interpolation of the cumulative frequency distribution of daily flows on to 101 points (0 to 100 in steps of 1)</td>
<td>Divide by catchment area to get specific FDC ($\text{m}^3 \text{s}^{-1} \text{km}^{-2}$)</td>
<td>Log base 10</td>
</tr>
<tr>
<td>FDC$_{\text{Feb}}$</td>
<td>Probability distribution of daily flow for February</td>
<td>Interpolation of the cumulative frequency distribution of daily flows for each calendar month on to 101 points (0 to 100 in steps of 1)</td>
<td>Divide by catchment area to get specific FDC ($\text{m}^3 \text{s}^{-1} \text{km}^{-2}$)</td>
<td>Log base 10</td>
</tr>
</tbody>
</table>
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<table>
<thead>
<tr>
<th>Defining characteristic</th>
<th>Categories</th>
<th>Notation</th>
<th>Category membership criteria</th>
</tr>
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<tr>
<td>Climate</td>
<td>Warm-extremely-wet</td>
<td>WX</td>
<td>Warm: mean annual temperature &gt; 12°C</td>
</tr>
<tr>
<td></td>
<td>Warm-wet</td>
<td>WW</td>
<td>Cool: mean annual temperature &lt; 12°C</td>
</tr>
<tr>
<td></td>
<td>Warm-dry</td>
<td>WD</td>
<td>Extremely Wet: mean annual effective precipitation(a) &gt; 1500 mm</td>
</tr>
<tr>
<td></td>
<td>Cool-extremely-wet</td>
<td>CX</td>
<td>Wet: mean annual effective precipitation &gt; 500 and &lt; 1500 mm</td>
</tr>
<tr>
<td></td>
<td>Cool-wet</td>
<td>CW</td>
<td>Dry: mean annual effective precipitation ≤ 500 mm</td>
</tr>
<tr>
<td></td>
<td>Cool-dry</td>
<td>CD</td>
<td></td>
</tr>
<tr>
<td>Topography</td>
<td>Glacial-mountain</td>
<td>GM</td>
<td>GM: M and % permanent ice &gt; 1.5%</td>
</tr>
<tr>
<td></td>
<td>Mountain</td>
<td>M</td>
<td>M: &gt; 50% annual rainfall volume above 1000 m ASL</td>
</tr>
<tr>
<td></td>
<td>Hill</td>
<td>H</td>
<td>H: 50% rainfall volume between 400 and 1000 m ASL</td>
</tr>
<tr>
<td></td>
<td>Low-elevation</td>
<td>L</td>
<td>L: 50% rainfall below 400 m ASL</td>
</tr>
<tr>
<td></td>
<td>Lake</td>
<td>Lk</td>
<td>Lk: Lake influence index(b) &gt; 0.033</td>
</tr>
</tbody>
</table>

\(a\). Effective precipitation = annual rainfall − annual potential evapotranspiration

\(b\). See Snelder and Biggs (2002) for a description.
Table 4. Codes and descriptions of independent variables used to fit regression models. See Leathwick et al., (2011) for full descriptions.

<table>
<thead>
<tr>
<th>Variable name</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>usPET_Q</td>
<td>Annual potential evapotranspiration of catchment (mm)</td>
</tr>
<tr>
<td>usRainDays10_Q</td>
<td>Catchment rain days, greater than 10 mm/month (days/year)</td>
</tr>
<tr>
<td>usAnRainVar_Q</td>
<td>Coefficient of variation of annual catchment rainfall (m)</td>
</tr>
<tr>
<td>usSteep_Q</td>
<td>% annual runoff volume from area of catchment with slope &gt; 30° (%)</td>
</tr>
<tr>
<td>usCatElev</td>
<td>Average elevation in the upstream catchment (m)</td>
</tr>
<tr>
<td>usParticleSize_Q</td>
<td>Catchment average of particle size (ordinal scale)</td>
</tr>
</tbody>
</table>
Table 5. Various metrics quantifying correspondence between observed and predicted values for four hydrological indices (Table 2) using various estimation methods.

<table>
<thead>
<tr>
<th>Index</th>
<th>Method</th>
<th>n</th>
<th>NSE</th>
<th>pbias</th>
<th>RSR</th>
</tr>
</thead>
<tbody>
<tr>
<td>(\log(Q_{\text{bar}}/\text{area}))</td>
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<td>0.446</td>
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<td>-0.410</td>
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<td>0.09</td>
<td>11.733</td>
<td>0.955</td>
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<td>0.08</td>
<td>2.499</td>
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<td>17.496</td>
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<td>-31.733</td>
<td>0.734</td>
</tr>
</tbody>
</table>

* Table footnote: In this comparison HUC estimates of instantaneous \(Q_{e}\) were compared with observed \(Q_{e}\) calculated from mean daily flow data.